

RIVAS Final Conference
“Vibrations – Ways out of the annoyance”
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Stiff wave barriers for the mitigation of railway induced vibrations

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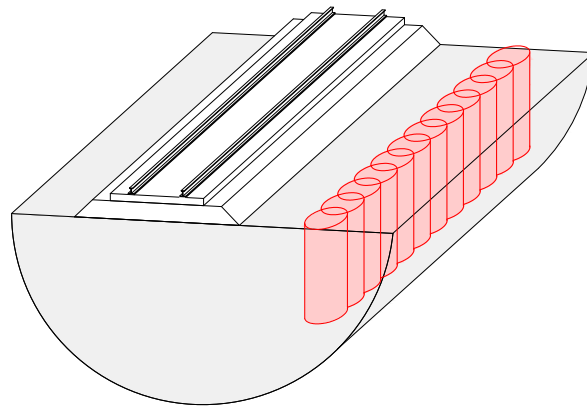
³ CEDEX, Spain

⁴ Keller Cimentaciones, Spain

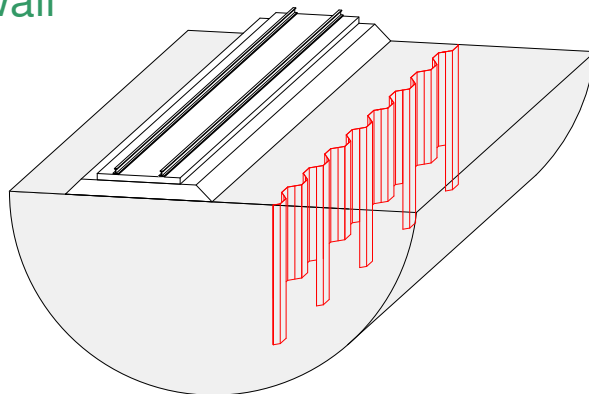
⁵ Trafikverket, Sweden

Stiff wave barriers

- Two types of stiff wave barriers are considered in this presentation:
 - Jet grouting wall



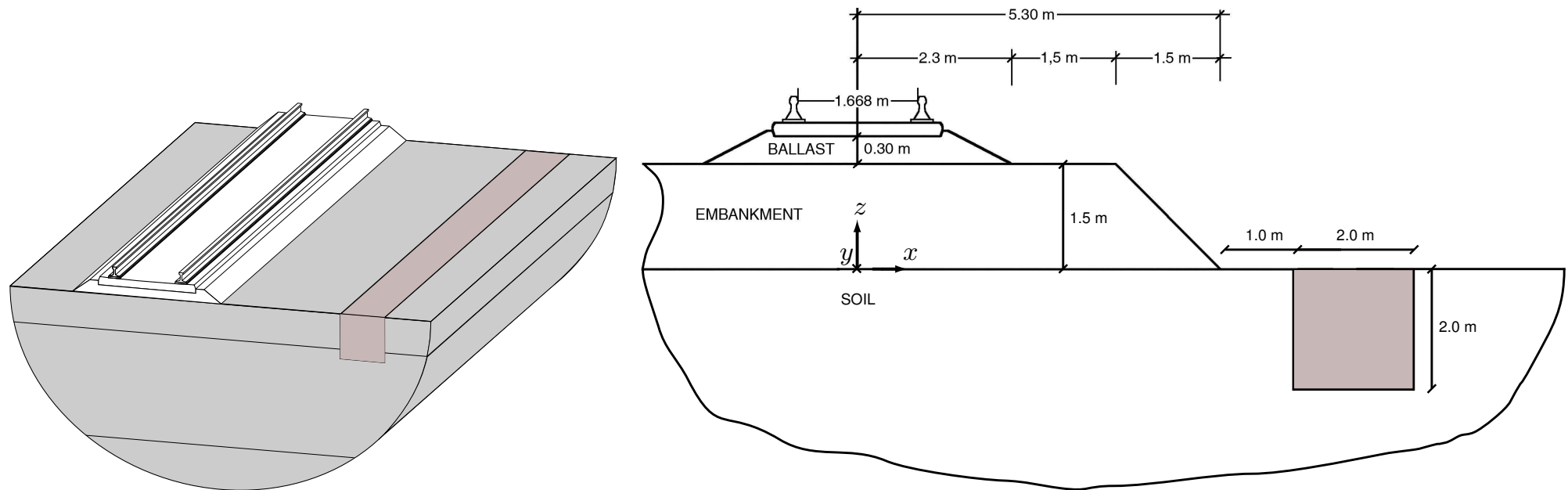
- Sheet piling wall



Jet grouting wall

Stiff wave barrier next to the track

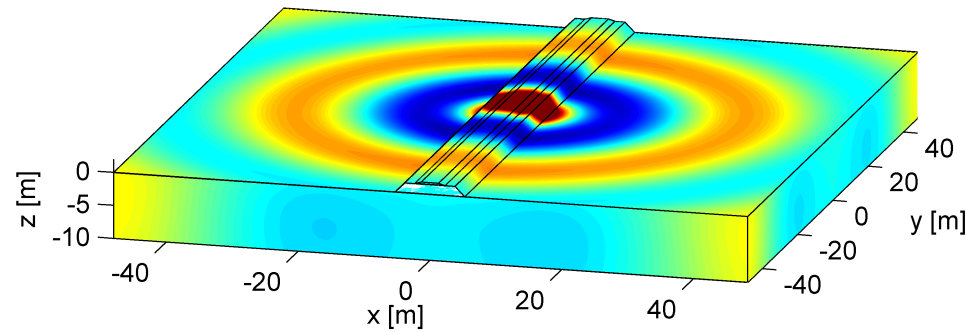
- Jet grouting wall ($w = h = 2$ m) in a homogeneous halfspace



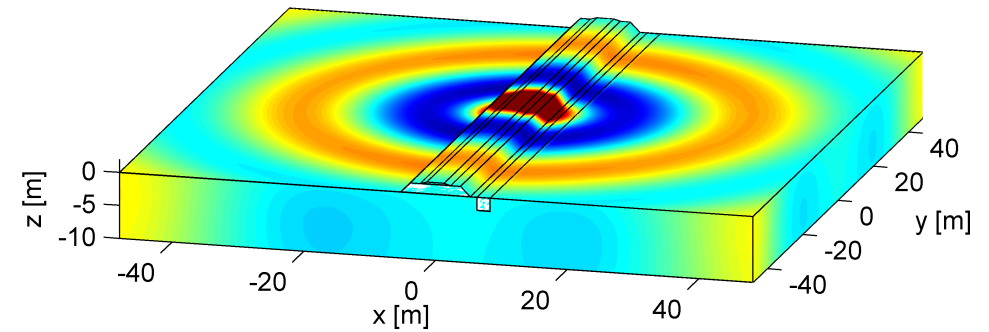
	C_s [m/s]	C_p [m/s]	β_s [—]	β_p [—]	ρ [kg/m ³]
Halfspace	200	400	0.025	0.025	2000
Stiffened soil	550	950	0.05	0.05	2000

Transfer functions

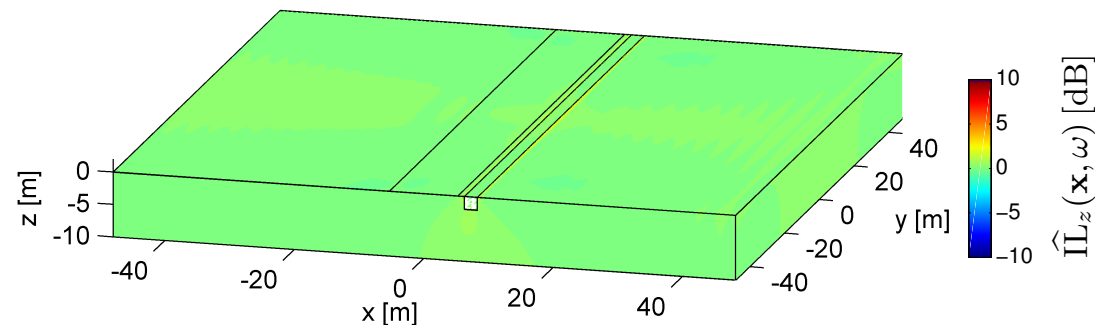
- Real part of the vertical displacement field $\hat{u}_z(\mathbf{x}, \omega)$ at 5 Hz
without barrier



with barrier

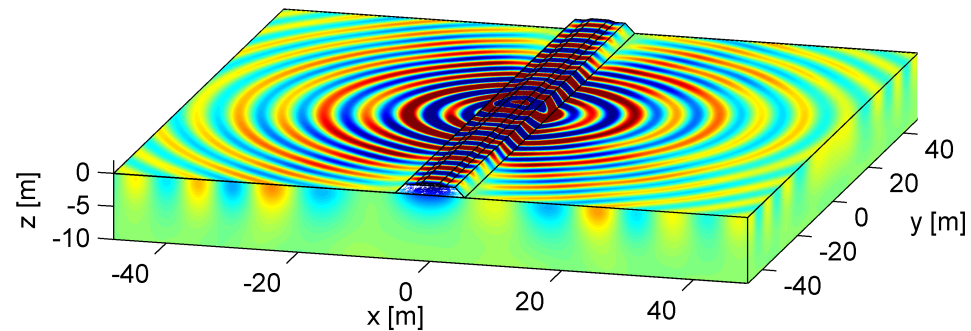


- Corresponding insertion loss $\hat{\text{IL}}_z(\mathbf{x}, \omega) = 20 \log_{10} \frac{|\hat{u}_z^{\text{ref}}(\mathbf{x}, \omega)|}{|\hat{u}_z(\mathbf{x}, \omega)|}$

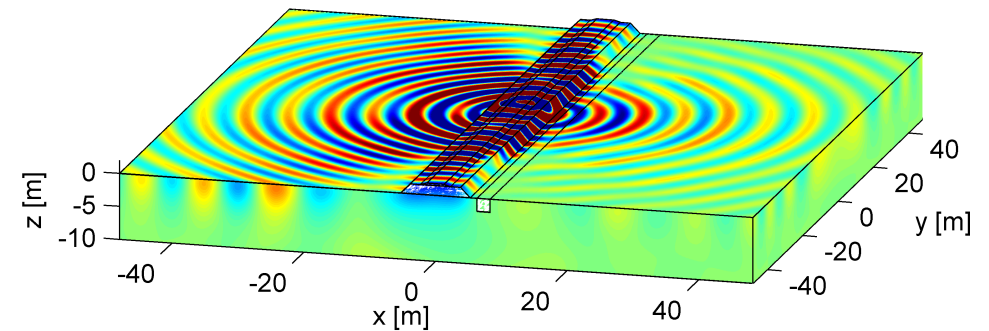


Transfer functions

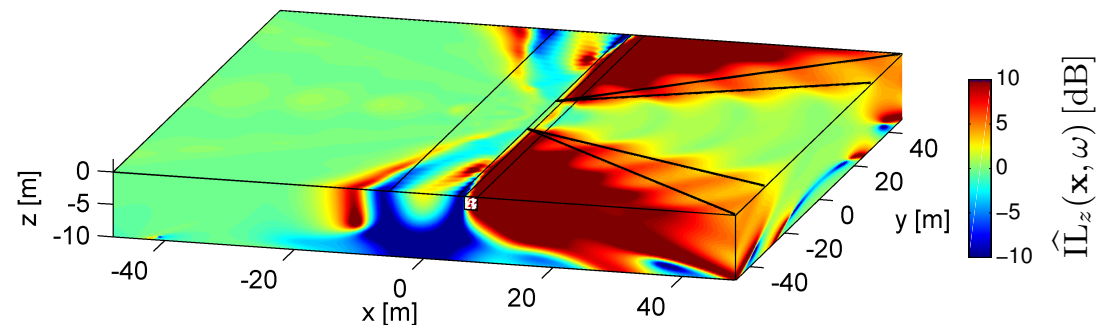
- Real part of the vertical displacement field $\hat{u}_z(\mathbf{x}, \omega)$ at 30 Hz
without barrier



with barrier

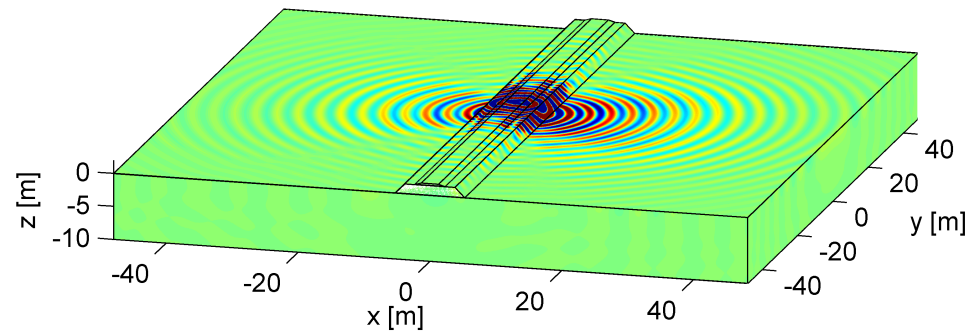


- Corresponding insertion loss $\hat{\text{IL}}_z(\mathbf{x}, \omega) = 20 \log_{10} \frac{|\hat{u}_z^{\text{ref}}(\mathbf{x}, \omega)|}{|\hat{u}_z(\mathbf{x}, \omega)|}$

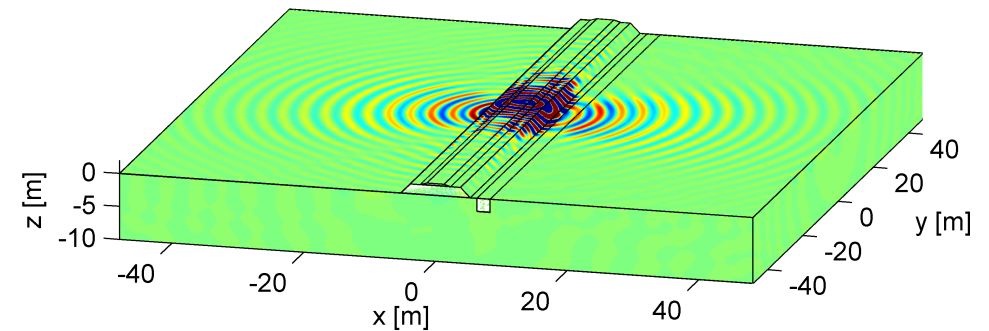


Transfer functions

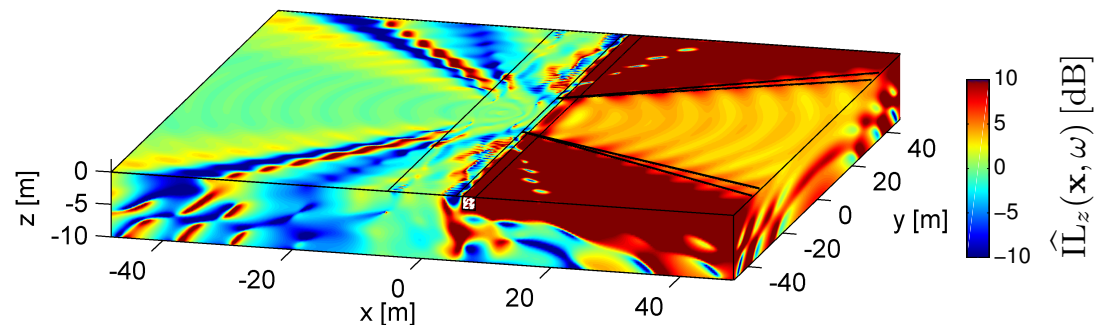
- Real part of the vertical displacement field $\hat{u}_z(\mathbf{x}, \omega)$ at 60 Hz
without barrier



with barrier

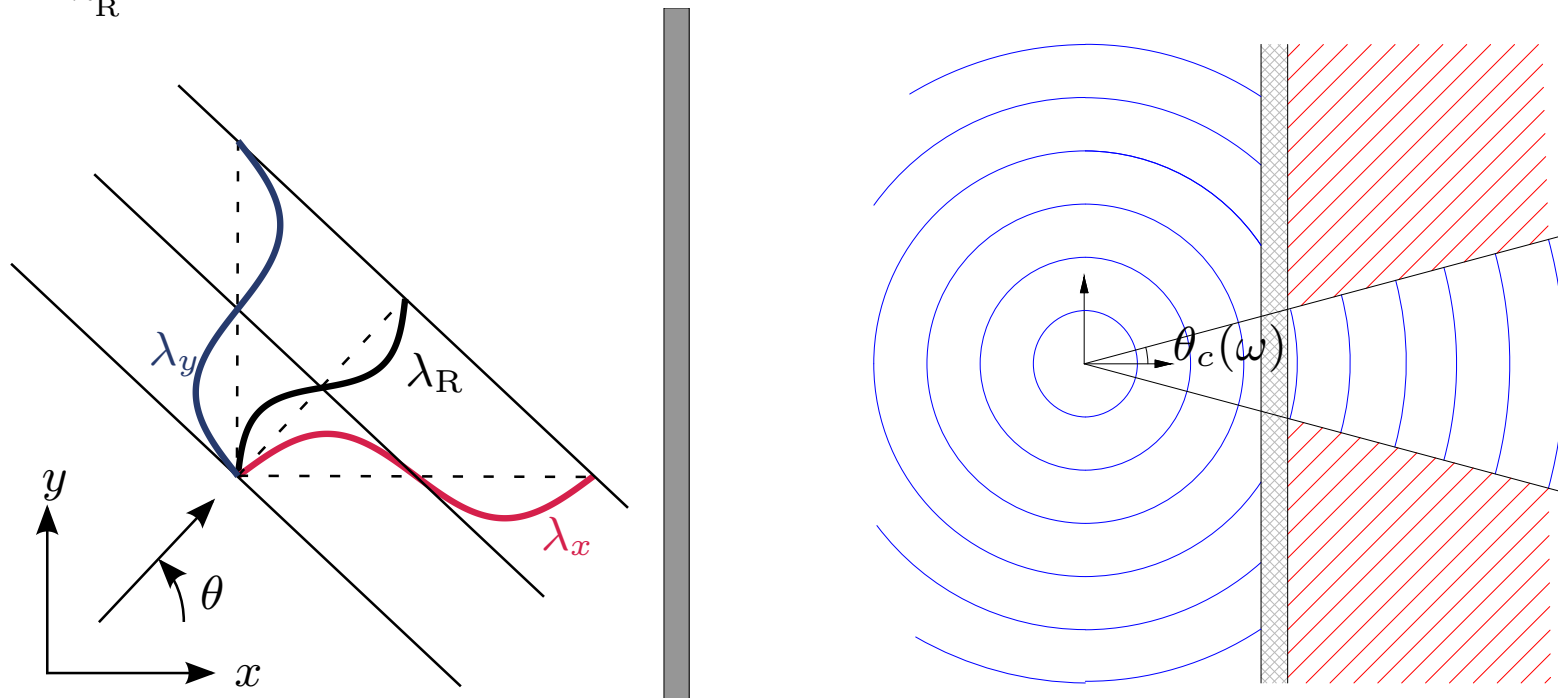


- Corresponding insertion loss $\hat{\text{IL}}_z(\mathbf{x}, \omega) = 20 \log_{10} \frac{|\hat{u}_z^{\text{ref}}(\mathbf{x}, \omega)|}{|\hat{u}_z(\mathbf{x}, \omega)|}$



Interaction of Rayleigh waves in the soil and bending waves in the stiff wave barrier

- Cylindrical wavefield can be decomposed into plane waves, satisfying the dispersion relation $\frac{1}{\lambda_x^2} + \frac{1}{\lambda_y^2} = \frac{1}{\lambda_R^2}$, where $\lambda_R = 2\pi \frac{C_R}{\omega}$ is the Rayleigh wavelength.



- Propagating plane waves are characterized by $\lambda_R \leq \lambda_y \leq \infty$:
 - $\theta = 0 \Rightarrow \lambda_x = \lambda_R, \lambda_y = \infty$
 - $\theta = \pi/2 \Rightarrow \lambda_y = \lambda_R, \lambda_x = \infty$

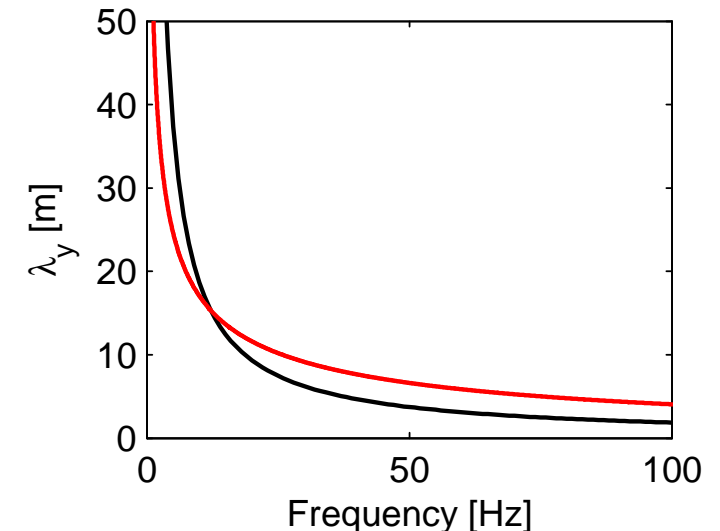
Interaction of Rayleigh waves in the soil and bending waves in the stiff wave barrier

- Rayleigh wave dispersion curve (black line):

$$\lambda_R = 2\pi \frac{C_R}{\omega}$$

- Free bending wave dispersion curve (red line):

$$\lambda_b = \frac{2\pi}{\sqrt{\omega}} \left(\frac{EI}{\rho A} \right)^{1/4}$$



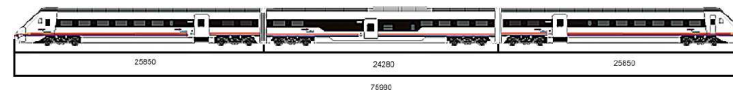
- $f < f_c$: $\lambda_b < \lambda_R \Rightarrow$ Rayleigh waves propagate unhindered through the block of stiffened soil
- $f > f_c$: $\lambda_b > \lambda_R \Rightarrow$ wavefield is partially transmitted, partially blocked
 - $\lambda_y > \lambda_b$: plane waves are transmitted
 - $\lambda_y < \lambda_b$: transmission of plane waves is impeded by the block of stiffened soil
- Critical frequency f_c :
- Critical angle $\theta_c(\omega) = \sin^{-1} (\lambda_R / \lambda_b)$:

$$f_c = \frac{C_R^2}{2\pi h} \sqrt{\frac{12\rho}{E}} = 12 \text{ Hz}$$

$$\sin \theta_c(\omega) = \frac{C_R}{\sqrt{\omega h}} \left(\frac{12\rho}{E} \right)^{1/4}$$

Train passage

Passage of a Renfe S599 train at 160 km/h



- Simplified model: only the unsprung masses are taken into account

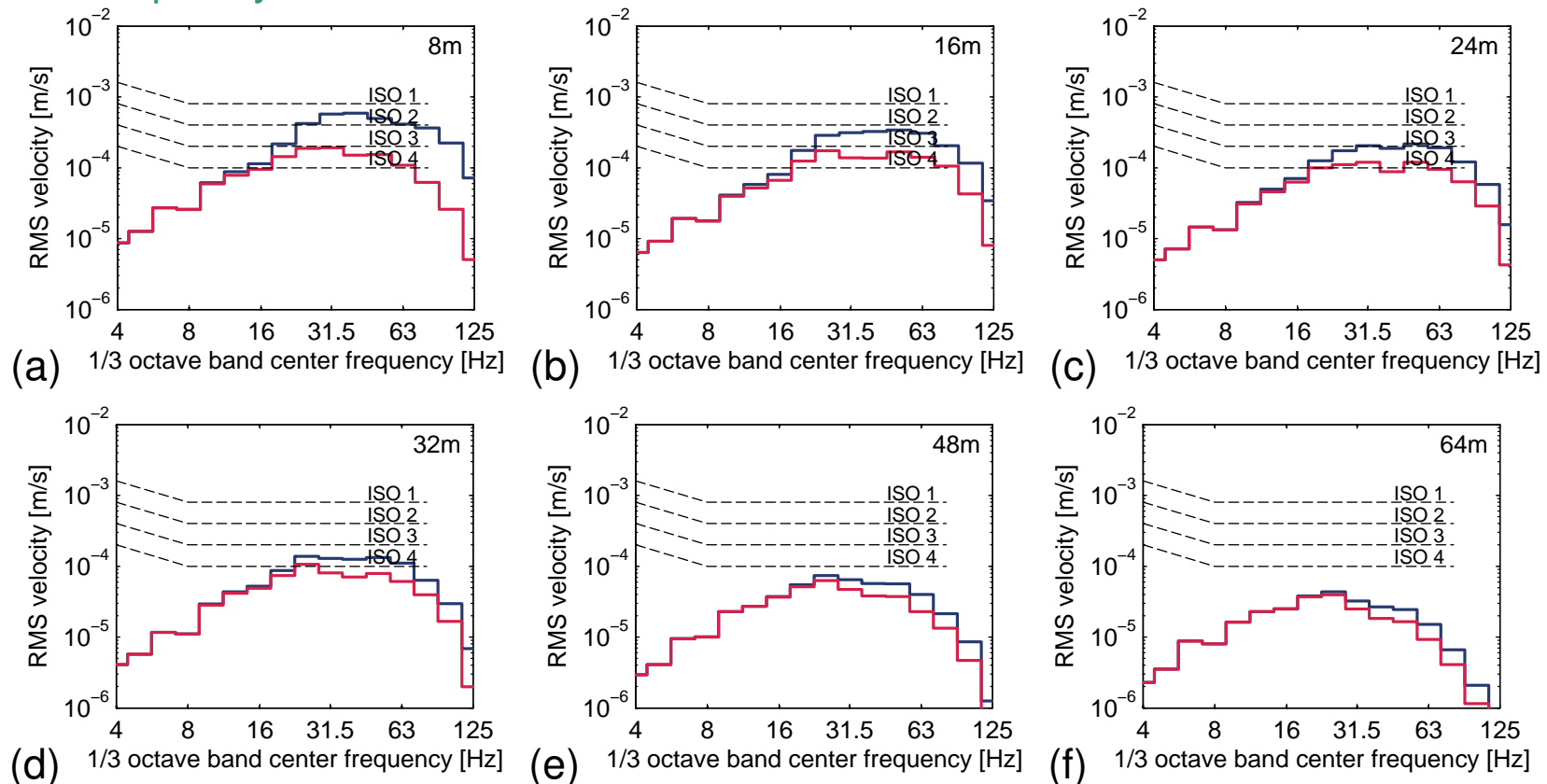
	N_a	L_t	L_b	L_a	M_u
	[—]	[m]	[m]	[m]	[kg]
Two motor coaches	4	25.85	17.73	2.50	1940
One central carriage	4	24.28	18.00	2.50	1704

- Hertzian contact spring ($k_{Hz} = 3 \times 10^9$ N/m); FRA track unevenness (class 3)

Train passage

Passage of a Renfe S599 train at 160 km/h

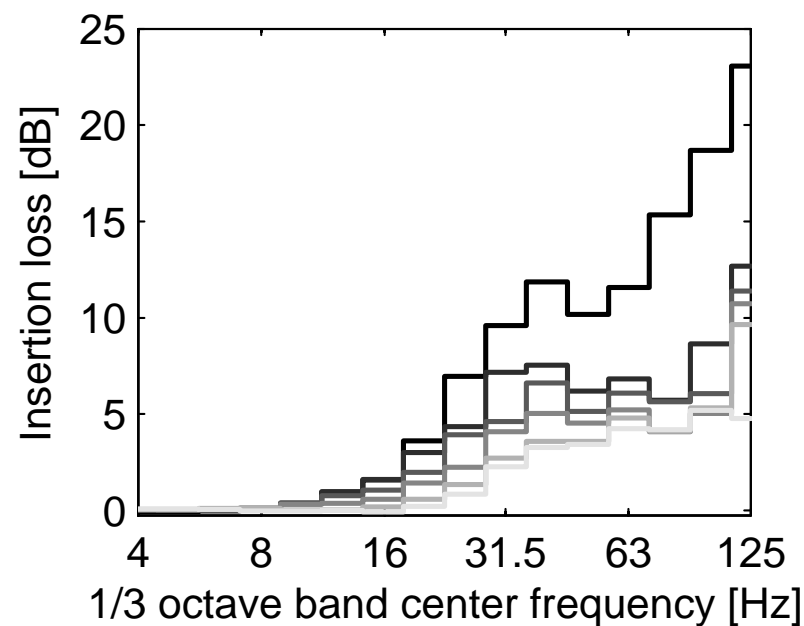
- One-third octave band RMS spectra of the vertical free field vibrations in the reference case (blue lines) and in case of stiffening next to the track (red lines)
- In case of subgrade stiffening next to the track, a reduction of vibration levels is obtained from the **critical frequency of 12 Hz** on.



Train passage

Passage of a Renfe S599 train at 160 km/h

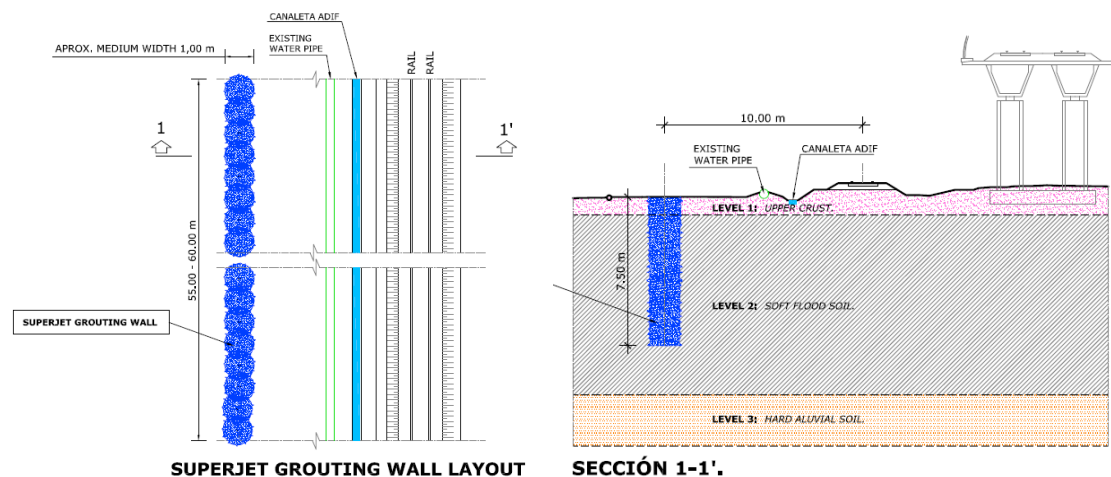
- Vertical insertion loss $\hat{\text{IL}}_z(\mathbf{x}, \omega)$ at 8 m, 16 m, 24 m, 32 m, 48 m and 64 m from the center of the track (black to light grey lines) due to the passage of a Renfe S599 at a speed of 160 km/h:
 - $\hat{\text{IL}}_z(x_1, \bar{\omega}) > \hat{\text{IL}}_z(x_2, \bar{\omega})$ with $x_1 < x_2$
 - $\hat{\text{IL}}_z(\bar{x}, \omega_1) < \hat{\text{IL}}_z(\bar{x}, \omega_2)$ with $\omega_1 < \omega_2$



Test site

El Realengo test site

- El Realengo test site in Spain: low Segura river flood plain.
- Conventional railway line (ADIF) between Murcia and Alicante
- A jet grouting wall of $1\text{ m} \times 55\text{ m} \times 7.5\text{ m}$ has been constructed



- An extensive measurement campaign will be carried out to verify the effectiveness of the jet grouting wall

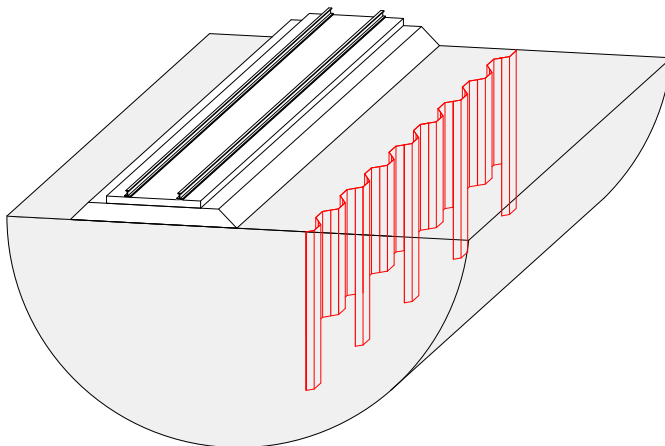
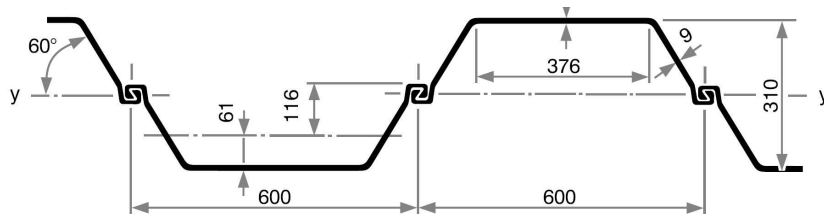
Sheet piling wall

Furet

- Located in southwest of Sweden along the West Coast Line between Gothenburg and Lund
- Vibration problems in houses nearby (4 – 5 Hz)

Sheet piling wall (100 m)

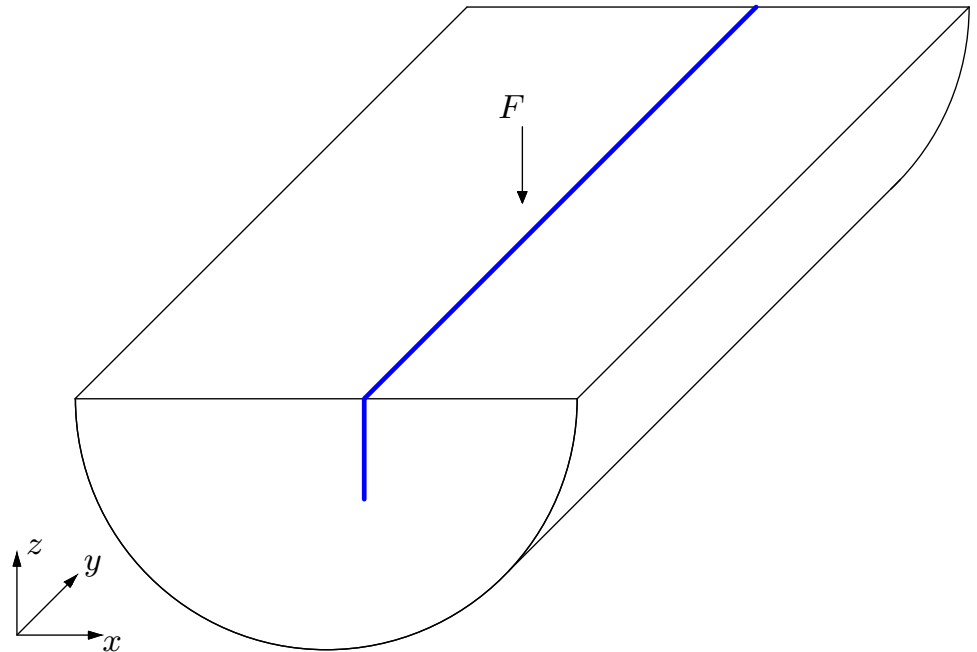
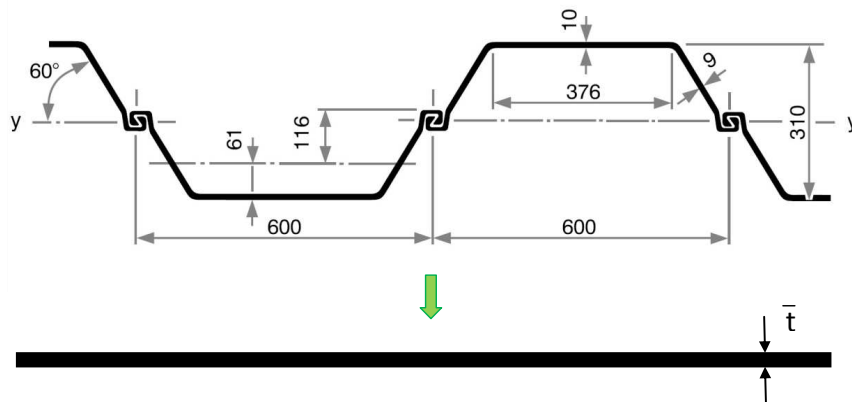
- VL 603-K profiles with depth of 12 m, every fourth pile extended to 18 m



Sheet piling wall model

2.5D methodology

- Longitudinally invariant geometry
 - Two models: depth 12 m and 18 m
 - Profiling



Equivalent orthotropic plate model

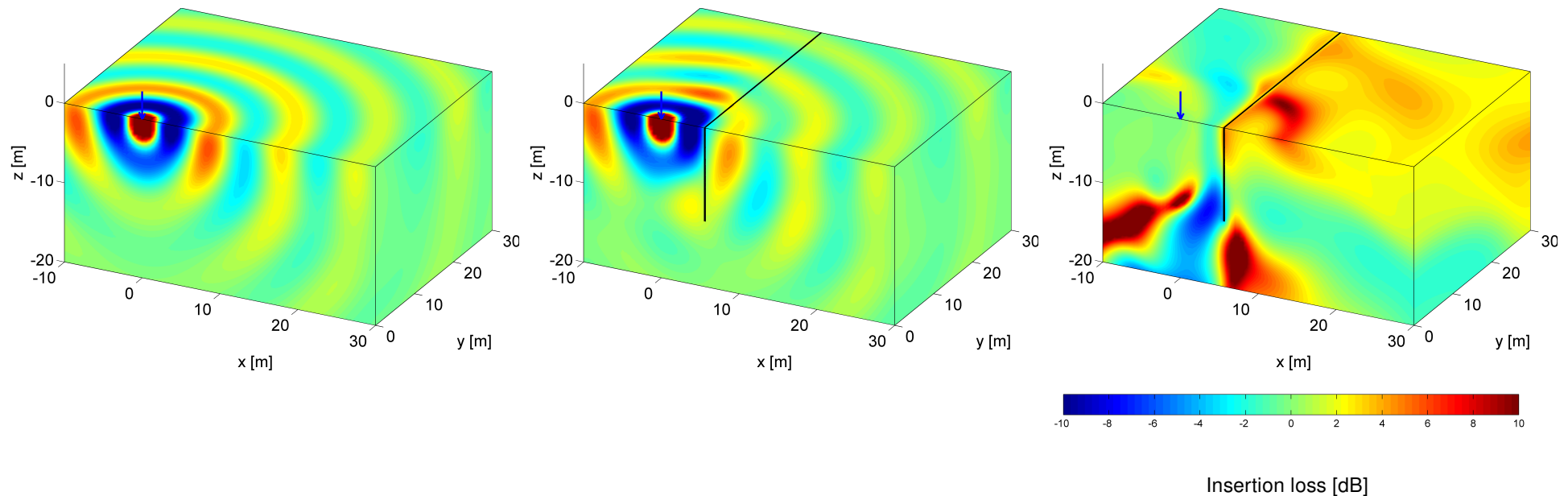
- Bending stiffness along the profiles B_z is much larger than bending stiffness perpendicular to the profiles B_y

Homogeneous halfspace

Dynamic soil characteristics (Horstwalde)

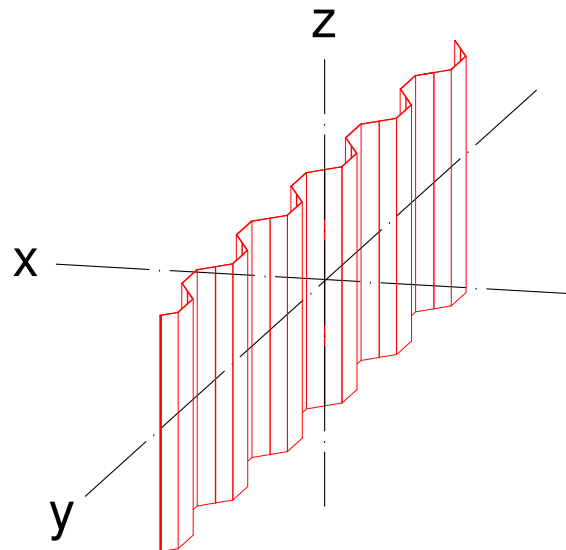
Layer	C_s [m/s]	C_p [m/s]	β_s [—]	β_p [—]	ρ [kg/m ³]	ν [—]
Halfspace	250	1470	0.025	0.025	1945	0.485

Vertical displacement and corresponding IL at 25 Hz



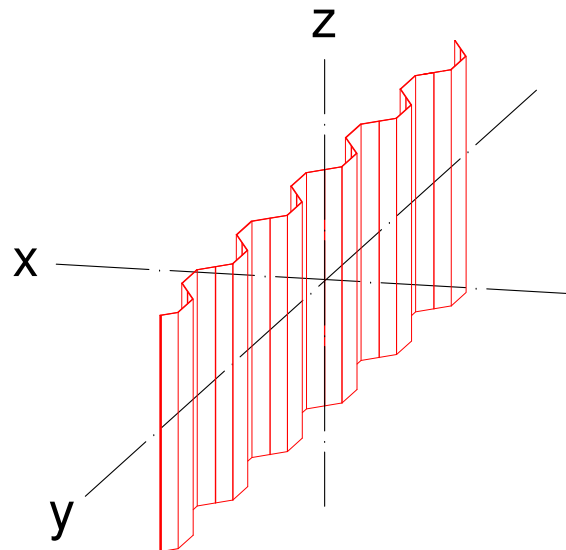
Wave impeding effect due to longitudinal bending stiffness

- Longitudinal bending waves with bending around x -axis are not excited due to limited width
- Longitudinal bending waves with bending around z -axis do not play a role due to low bending stiffness



Wave impeding effect due to longitudinal bending stiffness

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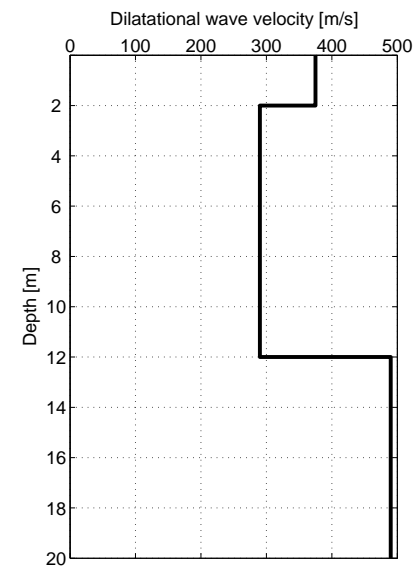
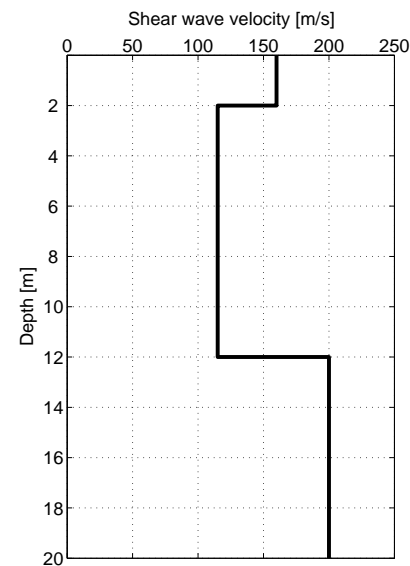
Wave impeding effect due to reflection of Rayleigh waves

- When the depth of the wave barrier is large enough compared to the Rayleigh wave length
- Rule of thumb: $d > 0.75\lambda_R$
- Almost independent of angle of incidence

Dynamic soil characteristics

- Geotechnical and geophysical surveys
- Soil profile

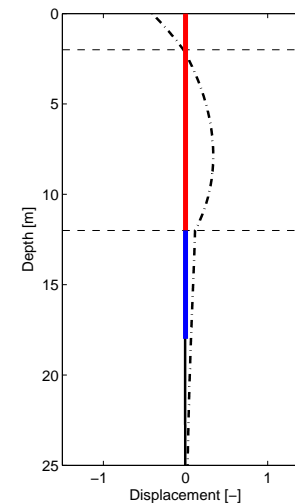
Layer	h [m]	C_s [m/s]	C_p [m/s]	β_s [—]	β_p [—]	ρ [kg/m ³]	ν [—]
1	2.0	154	375	0.025	0.025	1800	0.40
2	10.0	119	290	0.025	0.025	1850	0.40
Halfspace	∞	200	490	0.025	0.025	1710	0.40



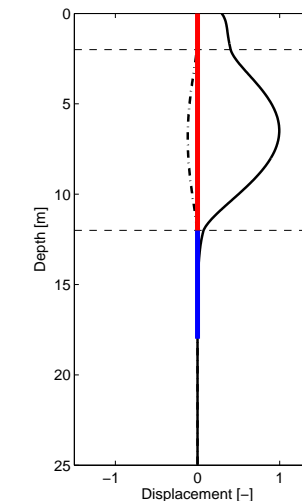
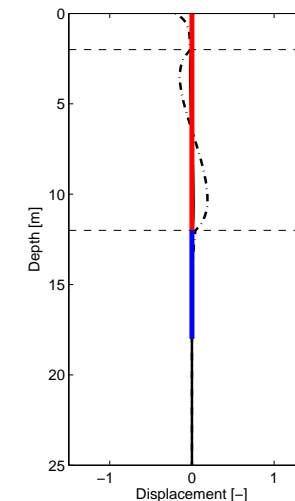
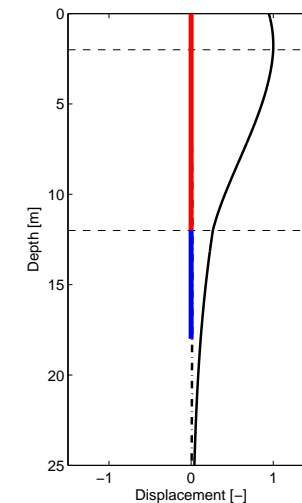
Fundamental Rayleigh wave

- Real (solid line) and imaginary (dashed-dotted line) part of the fundamental Rayleigh wave at 5 Hz
- Real (solid line) and imaginary (dashed-dotted line) part of the fundamental Rayleigh wave at 25 Hz

Horizontal component

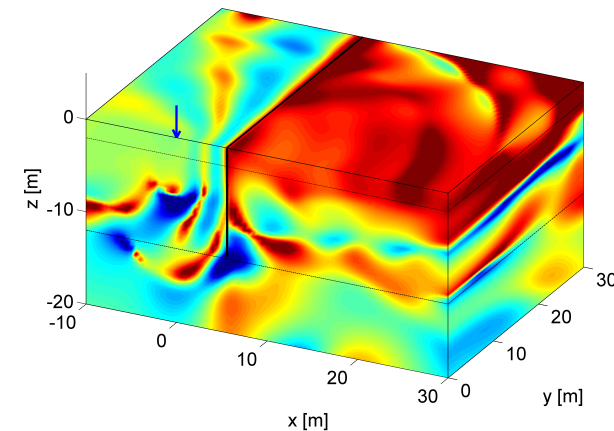
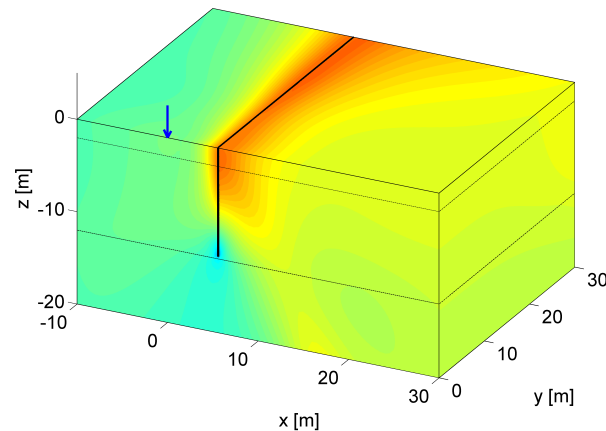


Vertical component

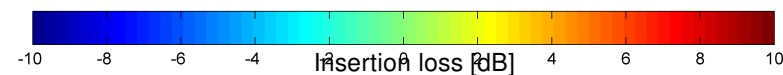
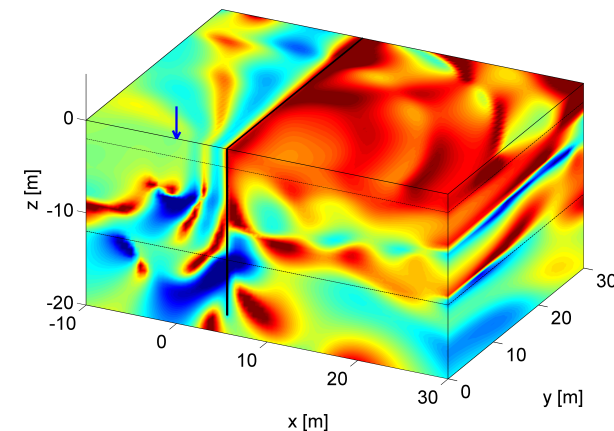
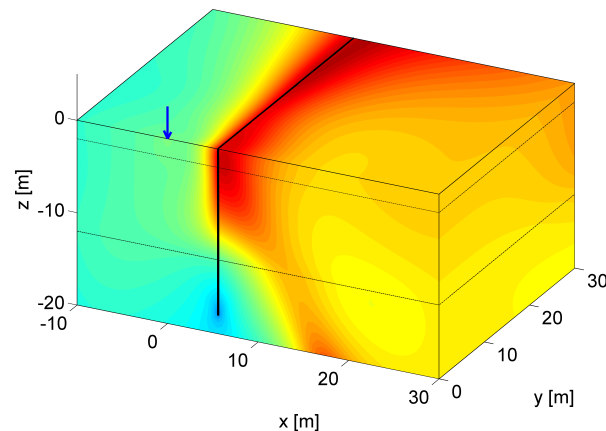


Furet test site

- Vertical insertion loss for a 12 m deep sheet pile wall at 5 Hz (left) and 25 Hz (right)



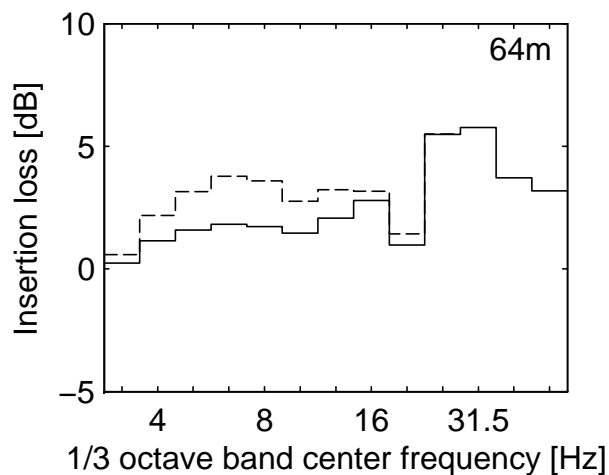
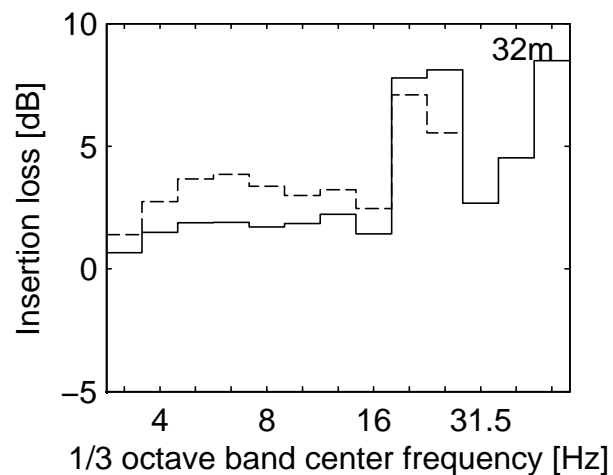
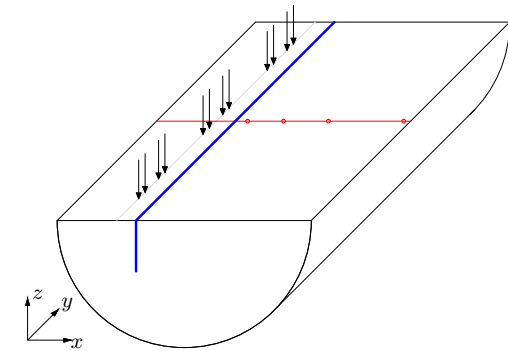
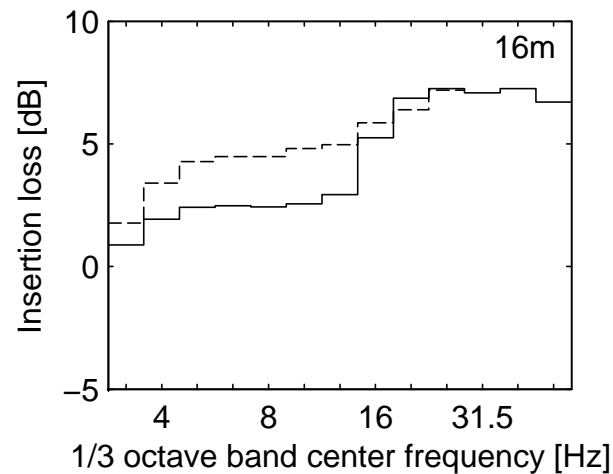
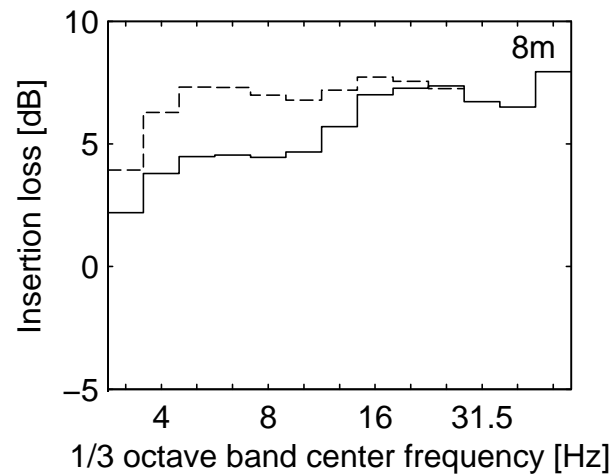
- Vertical insertion loss for a 18 m deep sheet pile wall at 5 Hz (left) and 25 Hz (right)



Furet test site



Vertical insertion loss for a line load



— depth 12 m
- - - depth 18 m

Conclusions

- Two phenomena contribute to the vibration reduction efficiency of a stiff wave barrier:
 - interaction between Rayleigh waves in the soil and bending waves in the barrier
(jet grouting wall)
 - reflections of Rayleigh waves if the barrier depth is large compared to the Rayleigh wavelength
(sheet piling wall)

Outlook

- Comparison between simulations and measurements (jet grouting wall and sheet piling wall)



- A design guide with practical recommendations (RIVAS Deliverable D4.6) will be made publicly available

Thank you for your attention

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